

Impact of Hinged Connectors on Sandwich Panel Behavior

Zach C. Ballard,¹; Ashley P. Thrall, A.M.ASCE²; Brian J. Smith, P.E.³; Casey M. Casias, S.M.ASCE⁴;

ABSTRACT

This paper discusses the impact of hinged connectors (a common connection in folding structures) on the behavior of sandwich panels (with fiber-reinforced polymer faces and foam core). A sandwich panel is subjected to uniform loading and tested when restrained by hinged connectors in compression and in tension. The measured results are compared to finite element numerical models, focusing on global behavior (displacements and strains at center) and local behavior (strains near connectors). Parametric studies using these validated numerical models investigate the impact of the number, size, and relative placement of hinged connectors. These studies culminate in guidelines for the design of structures comprised of hinged, folding panels. Ultimately, this paper addresses a research gap in understanding the behavior of sandwich panels connected by hinges.

CE Database subject headings: Sandwich panels; Hinges; Connections

INTRODUCTION

Sandwich panels are often used in shipping, aerospace, automotive, and construction industries where lightweight, high-strength materials are necessary. When connected by hinges, they can be utilized for folding, deployable structures where a small packaged volume and low self-weight are required for transportation (Quaglia et al., 2014).

¹Postdoctoral Research Associate, Kinetic Structures Laboratory, Department of Civil and Environmental Engineering and Earth Sciences, University of Notre Dame, Notre Dame, IN 46556. E-mail: zballard@nd.edu

²Myron and Rosemary Noble Assistant Professor of Structural Engineering, Kinetic Structures Laboratory, Department of Civil and Environmental Engineering and Earth Sciences, University of Notre Dame, Notre Dame, IN 46556. (corresponding author) E-mail: athrall@nd.edu

³Assistant Teaching Professor and Research Engineer, Kinetic Structures Laboratory, Department of Civil and Environmental Engineering and Earth Sciences, University of Notre Dame, Notre Dame, IN 46556. E-mail: bsmith24@nd.edu

⁴Graduate Student, Kinetic Structures Laboratory, Department of Civil and Environmental Engineering and Earth Sciences, University of Notre Dame, Notre Dame, IN 46556. E-mail: ccasias@nd.edu

19 A wide body of experimental research has been performed to better understand the properties
20 of isolated sandwich panels, including flexural strength [e.g., Manalo et al. (2010), Kesler and
21 Gibson (2002), Daniel and Abot (2000)], compressive strength [e.g., Malcom et al. (2013), Ma-
22 malis et al. (2005)], and characterization of failure modes [e.g., Russo and Zuccarello (2006)].
23 Fasteners/inserts play a key role in the strength and stiffness of sandwich panels and have been in-
24 vestigated both experimentally and numerically [e.g., Heimbs and Pein (2009), Bunyawanchakul
25 et al. (2005), Demelio et al. (2001), De Matteis and Landolfo (1999a)]. Despite the large num-
26 ber of research studies, the majority of past experimental work has been limited to understanding
27 the behavior of individual components. There have been few experimental or numerical studies
28 on structures comprised of multiple sandwich panels or panel-to-panel connections [e.g., Dawood
29 and Peirick III (2013), Heimbs and Pein (2009), De Matteis and Landolfo (1999a), De Matteis and
30 Landolfo (1999b)].

31 The objective of this research is to address the existing knowledge gap in understanding the
32 impact of hinged connectors on the behavior of structures comprised of sandwich panels. A single
33 sandwich panel [comprised of fiber-reinforced polymer (FRP) faces and a foam core] is experimen-
34 tally tested under a uniformly distributed surface pressure (emulating wind loads) when the sample
35 is restrained by hinged connectors in compression and in tension (as separate tests). The measured
36 results are compared to finite element numerical models, focusing on the panel displacements and
37 surface strains. These validated numerical models are used to perform parametric studies investi-
38 gating the impact of the number, size, and relative placement of hinged connectors. These studies
39 culminate in guidelines for the design of structures comprised of hinged, folding panels.

40 **MATERIAL PROPERTIES**

41 Material properties of the FRP face and foam core of the sandwich panel were measured ac-
42 cording to the applicable ASTM standards using an Instron 5590 Universal Testing Machine (Table
43 1). The 1.78 mm (0.07 in.) thick FRP faces are comprised of Vectorply biaxial (E-LT 1200-P) and
44 double-bias (E-BX 1200) e-glass laminate (layup: $0^\circ/90^\circ/45^\circ/-45^\circ/-45^\circ/45^\circ/90^\circ/0^\circ$) (Vectorply,
45 2002) with vinyl ester resin. The 31.8 mm (1.25 in.) core is Corecell M80 Foam (Gurit, 2013).

46 **EXPERIMENTAL PROGRAM**

47 A single sandwich panel was tested under two scenarios: (1) hinges in compression, where
48 the panel is restrained by hinges loaded in compression (Figure 1a-c) and (2) hinges in tension,
49 where the panel is restrained by hinges loaded in tension (Figure 1d-f). The panel was 1100 mm
50 (43.5 in.) long by 1070 mm (42 in.) wide. Panel end caps increased the face thickness to 4.95
51 mm (0.195 in.) near the panel edges. The panel featured three aluminum (alloy type 5052) hinges
52 along each transverse edge that were 76.2 mm (3.00 in) long and 2.54 mm (.100 in) thick with
53 a 6.35 mm (.250 in.) diameter pin, and an open leaf width of 76.2 mm (3.00 in.). Hinges were
54 placed at panel center and 114 mm (4.5 in.) from each edge in the transverse direction, inset 65.1
55 mm (2.56 in) from each edge in the longitudinal direction (Figure 2). In both tests, the panel was
56 aligned parallel to the ground at an approximate height of 127 mm (5.00 in.) and subjected to an
57 increasing uniformly distributed surface load designed to emulate wind pressure [up to 1.44 kPa
58 (30.0 psf)]. The load was applied to the surface of the panel via an urethane film air bladder [813
59 mm (32.0 in.) by 1120 mm (44.0 in.)] placed underneath the panel. For the hinges in compression
60 test, eyebolts connected the panel hinges to elevated W6x12 steel beams (Figure 1c) that served
61 as a rigid reaction frame. For the hinges in tension test, eyebolts were used to connect the panel
62 hinges to the floor via steel base plates (Figure 1f).

63 Midline panel displacements were measured by three displacement transducers (MD Totco
64 1850-002, string pots) attached to a W6x12 steel beam used as a stationary reference frame. Up to
65 18 strain gages (MicroMeasurements N2A-00-10CBE-350) were adhered to the panel to measure
66 longitudinal and transverse surface strains (Figure 2). The pressure from the air bladder on the
67 panel was measured using a pressure sensor (Omega PX409) placed in-line with the air tubes used
68 to inflate the bladder. Note that reported applied pressure refers to the pressure increase above the
69 internal air bladder pressure at full contact with the panel [at 0.96 kPa (20.0 psf)]. The correspond-
70 ing displacements and surface strains are reported. This does not include the displacements and
71 strains due to self-weight and that occurred during the uneven inflation of the air bladder prior to
72 full contact (since the degree of bladder contact during the inflation process could not be measured

73 or numerically simulated).

74 **NUMERICAL MODELING**

75 Three-dimensional finite element numerical models were developed using ABAQUS (ABAQUS,
76 2013). The FRP faces were modeled using S4R shell elements while the foam core was modeled
77 using C3D8R solid elements using a linear-elastic stress-strain relationship based on the measured
78 material properties (Table 1). Each face was continuously tied to the core surface. A single leaf for
79 each hinge was modeled as a rectangular aluminum (alloy type 5052) S4R shell element [assumed
80 material properties: $E=70.330$ MPa (10,200 ksi), $\rho=2680 \frac{kg}{m^3}$ ($168 \frac{lb}{ft^3}$)]. The hinge leaves were
81 tied to the panel end caps at three distinct nodes to match the fastener locations of each leaf to the
82 panel. A mesh size of 12.7 mm (0.500 in.) was used to ensure numerical convergence.

83 Boundary conditions were applied along lines located at the outer edge of each hinge, corre-
84 sponding to the location of the barrel (or rotation mechanism of the hinge). Models were created
85 for pin-roller, pin-pin, and fix-fix boundary conditions applied along this restraint line. The pin-
86 roller and pin-pin models were used to assess the observed relative translation permitted in the
87 hinged connectors and the fix-fix boundary conditions were used to investigate the effects of long-
88 term use in which hinges may become locked due to debris or corrosion. Note that the model for
89 the hinges in compression test features the hinges on the tension (i.e., top) face of the panel while
90 the model for the hinges in tension test features the hinges on the compression (i.e., bottom) face
91 of the panel. A uniformly distributed upward pressure was applied to the entire surface of the panel
92 to emulate the applied pressure from the air bladder after the bladder was fully in-contact with the
93 panel.

94 **RESULTS**

95 **Hinges in Compression Test**

96 Figures 3a and 3b show a comparison of the measured global (displacements and strains at cen-
97 ter) behavior of the hinges in compression test with numerical models featuring pin-roller, pin-pin,
98 and fix-fix hinge barrel boundary conditions. The measured displacements and the longitudinal

99 surface strains at the center of the panel closely match the pin-roller hinge barrel boundary con-
100 dition model, indicating that hinges permit some horizontal translation of the panel (i.e., internal
101 movement of the barrel). If this horizontal translation becomes limited in the field (pin-pin con-
102 ditions) or if rotation is also restrained (fix-fix condition), the global load-displacement behavior
103 becomes stiffer.

104 Considering the local behavior (strains near connectors), the pin-roller hinge barrel boundary
105 condition provides an excellent prediction for the measured surface strains at all hinge locations
106 (Figure 3c). If a pin-pin hinge barrel boundary condition were to occur over long-term use due to
107 accumulation of debris or corrosion within the hinge, the strains are predicted to become compres-
108 sive and increase dramatically in magnitude (Figure 3c). If a fix-fix condition occurs, this effect
109 is observed to a slightly lesser degree. Based on these results, it is recommended that designers
110 evaluate hinges as pin-roller, pin-pin, and fix-fix conditions to obtain an envelope of possible local
111 strains. Additional reinforcing (e.g., thickening of FRP) in these regions may be warranted.

112 **Hinges in Tension Test**

113 Figures 4a and 4b show a comparison of the global measured behavior of the hinges in ten-
114 sion test with numerical models featuring pin-roller, pin-pin, and fix-fix hinge barrel boundary
115 conditions. The measured center displacements (Figure 4a) more closely resemble the pin-roller
116 model, while the longitudinal surface strains at the center of the panel (Figure 4b) are stiffer than
117 predictions from all three numerical models. This can be attributed to approximations in modeling
118 the core. The sample panel core is impregnated with small columns of vinyl ester resin during
119 manufacturing. This added stiffness is not accounted for in the numerical model. This effect is
120 more noticeable in the hinges in tension test since the supports are on the opposite face to the mea-
121 sured strain; therefore, the core plays a larger part in the behavior. Consistent with the hinges in
122 compression test, the fix-fix and pin-pin hinge barrel boundary conditions result in stiffer behavior.

123 As in the hinges in compression test, the most significant effect of the hinge barrel bound-
124 ary conditions can be seen in the localized longitudinal surface strains (Figure 4c). These strains
125 are measured on the same face as the support; therefore they are not significantly affected by the

126 modeling of the core. In general, the pin-roller hinge barrel boundary condition provides an ex-
127 cellent prediction for the measured surface strains. The pin-pin condition changes the strains from
128 compressive to tensile and significantly increases the magnitude (consistent with findings from the
129 hinges in compression test). A similar effect occurs in the fix-fix condition, though to a lesser ex-
130 tent. An exception occurs at Corner A - Right (upper right plot in Figure 4c) where the measured
131 results are between the pin-roller and the pin-pin/fix-fix conditions. This discrepancy is likely due
132 to the relative allowable movement in the hinge near Corner A - Right, which underscores the im-
133 portance of designers evaluating pin-roller, pin-pin, and fix-fix hinge barrel boundary conditions.

134 **PARAMETRIC STUDY OF THE NUMBER, SIZE, AND PLACEMENT OF HINGED** 135 **CONNECTORS**

136 The comparisons between the measured and numerical results validated the numerical models
137 for use in a parametric study assessing the impact of the number (Figure 5), size (Figure 6), and
138 relative placement (Figure 7) of hinged connectors on both global and local behavior (Ballard et al.,
139 2015). The hinges in compression numerical model was used for all results with pin-roller hinge
140 barrel boundary conditions since the numerical predictions more closely matched the measured
141 behavior than the hinges in tension model. Minor revisions to the numerical model to isolate the
142 effect of changes in the hinged connectors include (1) extending the longitudinal width of the end
143 caps to 116 mm (4.56 in.) along the full transverse length, and (2) continuously tying hinge leafs to
144 the surface as opposed to three discrete locations within the hinge leaf that corresponded to fastener
145 locations. To fully capture local behavior, the reported strain corresponds to the local maximum in
146 the end cap near the hinges, as this would be the region prone to failure.

147 **Impact of Number of Hinged Connectors**

148 Figure 5 compares the global behavior (i.e., midline and maximum displacements, Figure 5a
149 and 5b) and local behavior (i.e., longitudinal strains near hinges in the end cap, Figure 5c and 5d)
150 of varying the number of hinges. This parametric study included four models featuring from one
151 to four hinges [38.1 mm (1.50 in.) wide x 76.2 mm (3.00 in.) long x 2.54 mm (.100 in.) thick,

152 i.e., hinge size in sample] spaced approximately equidistant along each transverse edge, and a fifth
153 model featuring a single continuous hinge.

154 As expected, more hinges result in stiffer panel behavior as shown by decreased midline dis-
155 placements (Figure 5a). However, there is effectively no added benefit in increasing beyond 3
156 hinges (Figure 5b). Further, increasing the number of hinges, and especially the use of a continu-
157 ous hinge, reduces the magnitude of local compressive strain concentrations (Figure 5c). However,
158 there is again limited added benefit in implementing more than three hinges (Figure 5d). Based on
159 these results, it is recommended that a hinge-number to transverse-length ratio of approximately
160 3.0 per meter balances the benefits of improved behavior with the added expense and weight of
161 additional/continuous hinges.

162 **Impact of Size of Hinged Connectors**

163 Figure 6 shows the effect of hinge leaf length (see Figure 2 for definition) on (1) global behav-
164 ior indicated by midline and maximum displacements (Figure 6a and 6b) and (2) local behavior
165 indicated by longitudinal strains near hinges in the end cap (Figure 6c and 6d). Based on the above
166 discussed results, all numerical models in this subsection feature three hinge connectors on each
167 edge in the locations shown in Figure 2.

168 As expected, increasing the hinge length reduces the midline panel displacements (Figure 6a).
169 While the continuous hinge shows the smallest displacements, the difference between continuous
170 and the discrete hinges of varying lengths is insignificant in magnitude (Figure 6b). Local zones
171 of high compressive strains are reduced as the hinge leaf lengthens (Figure 6c). The magnitude of
172 this effect reduces with larger hinge lengths [i.e., the decrease in strain from the 76.2 mm (3.00
173 in.) to 152 mm (6.00 in.) hinge is less than that from 38.1 mm (1.50 in.) to 76.2 mm (3.00 in.)].
174 While longer hinge lengths, particularly the use of continuous hinges, improve local behavior,
175 there is additional cost and weight associated with this design decision. To balance these priorities,
176 designers should aim for a ratio of total-hinge-length (i.e., sum of lengths of all three hinges on
177 one edge) to transverse-length ratio of around 0.2 [approximately that of the 76.2 mm (3.00 in.)
178 hinge leaf investigated here].

179 The hinge width and thickness have negligible impact on the panel behavior [see Ballard et al.
180 (2015)]. Therefore, the hinge width should be limited to the dimension necessary for secure fas-
181 tening to the panel. Similarly, the hinge leaf thickness should be as thin as possible while meeting
182 the required demands.

183 **Impact of Relative Placement of Hinged Connectors**

184 Figure 7 shows the impact of varying the distance between the center hinge and the outer hinges
185 (identified in Figure 2) on the panel behavior. Based on the above discussed results, each model
186 includes three hinges per side, with each hinge being 38.1 mm (1.50 in.) wide x 76.2 mm (3.00
187 in.) long x 2.54 mm (.100 in.) thick.

188 As expected, as the hinges are spaced further apart, the maximum midline panel deflections
189 become smaller and deflections along the length of the panel become more uniform (Figure 7a and
190 7b). As the hinges move to the center, the displacement significantly increases, especially at the
191 panel ends. Similarly, the surface compressive strains are reduced as the hinges are spaced further
192 apart (Figure 7c), approximately linearly with the hinge distance (Figure 7d). For favorable panel
193 behavior, designers should aim to place hinges equidistant along the panel edge.

194 **Guidelines for Design**

195 Based on this parametric study, the following guidelines for the design of sandwich panels
196 connected by hinges are recommended:

- 197 ● Select a hinge-number to transverse-length ratio of approximately 3.0 per meter to balance
198 improved performance with the cost/weight of hinges.
- 199 ● Select a hinge-length to transverse-length ratio of 0.2 to balance improved performance
200 with cost/weight of hinges.
- 201 ● Limit hinge width to that needed for secure fastening to the panel.
- 202 ● Limit hinge thickness to meet the required demands.
- 203 ● Space hinges equidistant for smaller and more uniform panel displacements with reduced
204 local surface strains

205 These design guidelines are intended as qualitative guidelines for preliminary design. A detailed
206 analysis and design would be required for a specific project.

207 **CONCLUSIONS**

208 This paper discussed the impact of hinged connectors (a common connection in folding struc-
209 tures) on the behavior of a representative sandwich panel (comprised of FRP faces and a foam
210 core). The sandwich panel was subjected to a uniformly applied distributed load emulating wind
211 pressures. Both experimental and numerical studies (using finite element models created in ABAQUS)
212 were conducted to better understand the global and local panel behavior, with measured and pre-
213 dicted comparisons focusing on the panel displacements and longitudinal surface strains. Validated
214 numerical models were then used to perform parametric studies, culminating in qualitative guide-
215 lines for design. Overall, this paper addresses a research gap in understanding the behavior of
216 sandwich panels that feature hinged connectors.

217 **ACKNOWLEDGEMENTS**

218 This material is based upon work supported by the US Army Natick Soldier Research, Devel-
219 opment and Engineering Center (NSRDEC) under Contract W911QY-12-C-0128.

220 **REFERENCES**

- 221 ABAQUS (2013). “ABAQUS/Standard Analysis User’s Manual Version 6.13. Dassault Systemes,
222 Waltham, MA.
- 223 Ballard, Z. C., Thrall, A. P., and Smith, B. J. (2015). “Parametric study of the effect of hinged con-
224 nectors on the behavior of origami-inspired structures comprised of sandwich panels.” *Proceed-*
225 *ings of the ASME 2015 International Design Engineering Technical Conferences & Computers*
226 *and Information in Engineering Conference*. Boston, MA.
- 227 Bunyawanichakul, P., Castanie, B., and Barrau, J. J. (2005). “Experimental and numerical analysis
228 of inserts in sandwich panels.” *Applied Composite Materials*, 12(3-4), 177–191.

- 229 Daniel, I. M. and Abot, J. L. (2000). “Fabrication, testing and analysis of composite sandwich
230 beams.” *Composites Science and Technology*, 60(12-13), 2455–2463.
- 231 Dawood, M. and Peirick III, L. (2013). “Connection development and in-plane response of glass
232 fiber reinforced polymer sandwich panels with reinforced cores.” *Canadian Journal of Civil
233 Engineering*, 40(11), 1117–1126.
- 234 De Matteis, G. and Landolfo, R. (1999a). “Mechanical fasteners for cladding sandwich panels:
235 Interpretative models for shear behaviour.” *Thin-Walled Structures*, 35(1), 61–79.
- 236 De Matteis, G. and Landolfo, R. (1999b). “Structural behaviour of sandwich panel shear walls:
237 An experimental analysis.” *Materials and Structures*, 32(219), 331–341.
- 238 Demelio, G., Genovese, K., and Pappalettere, C. (2001). “An experimental investigation of static
239 and fatigue behavior of sandwich composite panels joined by fasteners.” *Composites Part B:
240 Engineering*, 32(4), 299–308.
- 241 Gurit (2013). “Corecell M-foam - the marine foam. [http :
242 //www.gurit.com/files/documents/corecell – m – foamv5pdf.pdf](http://www.gurit.com/files/documents/corecell-m-foamv5pdf.pdf) (October 9, 2013).
- 243 Heimbs, S. and Pein, M. (2009). “Failure behavior of honeycomb sandwich corner joints and
244 inserts.” *Composite Structures*, 89(4), 575–588.
- 245 Kesler, O. and Gibson, L. J. (2002). “Size effects in metallic foam core sandwich beams.” *Materials
246 Science and Engineering A*, A326, 228–234.
- 247 Malcom, A. J., Aronson, M. T., Deshpande, V. S., and Wadley, H. N. G. (2013). “Compressive
248 response of glass fiber composite sandwich structures.” *Composites Part A: Applied Science and
249 Manufacturing*, 54, 88–97.
- 250 Mamalis, A. G., Manolakos, D. E., Ioannidis, M. B., and Papapostolou, D. P. (2005). “On the
251 crushing response of composite sandwich panels subjected to edgewise compression: Experi-
252 mental.” *Composite Structures*, 71(2), 246–257.

- 253 Manalo, A. C., Aravinthan, T., Karunasena, W., and Islam, M. M. (2010). “Flexural behaviour
254 of structural fibre composite sandwich beams in flatwise and edgewise positions.” *Composite
255 Structures*, 92(4), 984–995.
- 256 Quaglia, C. P., Dascanio, A. J., and Thrall, A. P. (2014). “Bascule shelters: A novel erection
257 strategy for origami-inspired structures.” *Engineering Structures*, 75, 276–287.
- 258 Russo, A. and Zuccarello, B. (2006). “Experimental and numerical evaluation of the mechanical
259 behaviour of GFRP sandwich panels.” *Composite Structures*, 81(4), 575–586.
- 260 Vectorply (2002). “Laminated properties and packaging. [http : //www.vectorply.com/ri –
261 laminateprop.html](http://www.vectorply.com/ri-laminateprop.html) (November 29, 2014).

262 **List of Tables**

263 1 Measured material properties of sandwich panel components. 13

TABLE 1: Measured material properties of sandwich panel components.

Property	Panel Core					Panel Face			
	ρ_c (kg/m ³)	E_c (MPa)	σ_c (MPa)	τ_c (MPa)	G_c (MPa)	ρ_f (kg/m ³)	E_f (MPa)	σ_f (MPa)	ν_f
Mean	87.5	57.7	1.25	1.56	47.2	1740	15500	283	0.261
Std. Dev.	0.833	2.44	9.86e-3	9.45e-3	2.16	5.20	737	17.0	0.0145
COV	0.950%	4.23%	0.790%	6.03%	4.59%	0.299%	4.76%	6.02%	5.55%
ASTM Standard	C271	C365		C393	D7250	D792	D3039		
No. of Samples	10	10	10	5	5	5	8	5	8

Note: Subscripts *c* and *f* correspond to sandwich panel core and face, respectively. ρ = density, E = elastic modulus, G = shear modulus, τ = shear strength, σ = ultimate strength, ν = Poisson's ratio, Std. Dev. = standard deviation, and COV = coefficient of variation.

264 **List of Figures**

265 1 Hinges in compression test: (a) elevation view, (b) photograph, and (c) hinged
 266 connector photograph; Hinges in tension test: (d) elevation view, (e) photograph
 267 and (f) hinged connector photograph. 15

268 2 Plan view of measurement system locations for (a) hinges in compression test and
 269 (b) hinges in tension test. 16

270 3 Measured and numerical behavior (tension face) for hinges in compression test:
 271 (a) global displacements (center of panel), (b) global longitudinal strains (center of
 272 panel), and (c) localized longitudinal strains near hinges. 17

273 4 Measured and numerical behavior for hinges in tension test: (a) global displace-
 274 ments (tension face, center of panel), (b) global longitudinal strains (tension face,
 275 center of panel), and (c) localized longitudinal strains near hinges (compression
 276 face). 18

277 5 Numerical behavior of panel (tension face) with varying number of hinged connec-
 278 tors: (a) global displacements along panel midline at an applied pressure of 0.479
 279 kPa, (b) maximum displacement at an applied pressure of 0.479 kPa, (c) longitudi-
 280 nal strain near hinge, and (d) maximum longitudinal strain near hinge at an applied
 281 pressure of 0.479 kPa. Reprinted from Ballard et al. (2015), permission request
 282 pending. 19

283 6 Numerical behavior of panel (tension face) with varying length of hinged connec-
 284 tors: (a) global displacements along panel midline at an applied pressure of 0.479
 285 kPa, (b) maximum displacement at an applied pressure of 0.479 kPa, (c) longitudi-
 286 nal strain near hinge, and (d) maximum longitudinal strain near hinge at an applied
 287 pressure of 0.479 kPa. Reprinted from Ballard et al. (2015), permission request
 288 pending. 20

289 7 Numerical behavior of panel (tension face) with varying hinge placement: (a)
 290 global displacements along panel midline at an applied pressure of 0.479 kPa, (b)
 291 maximum displacement at an applied pressure of 0.479 kPa, (c) longitudinal strain
 292 near hinge, and (d) maximum longitudinal strain near hinge at an applied pressure
 293 of 0.479 kPa. Reprinted from Ballard et al. (2015), permission request pending. . . 21

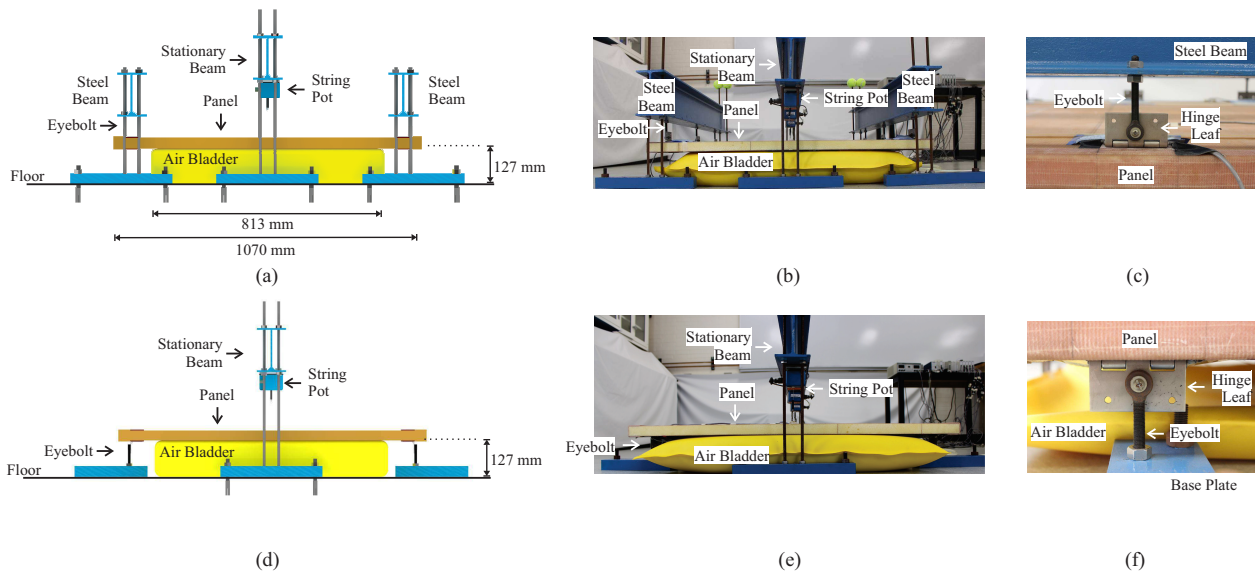


FIG. 1: Hinges in compression test: (a) elevation view, (b) photograph, and (c) hinged connector photograph; Hinges in tension test: (d) elevation view, (e) photograph and (f) hinged connector photograph.

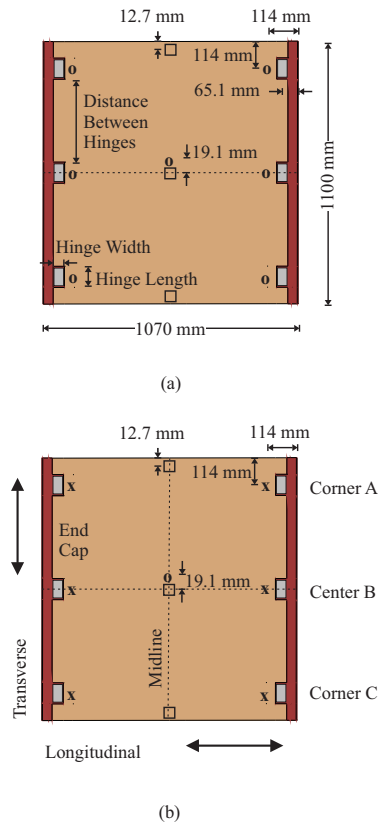


FIG. 2: Plan view of measurement system locations for (a) hinges in compression test and (b) hinges in tension test.

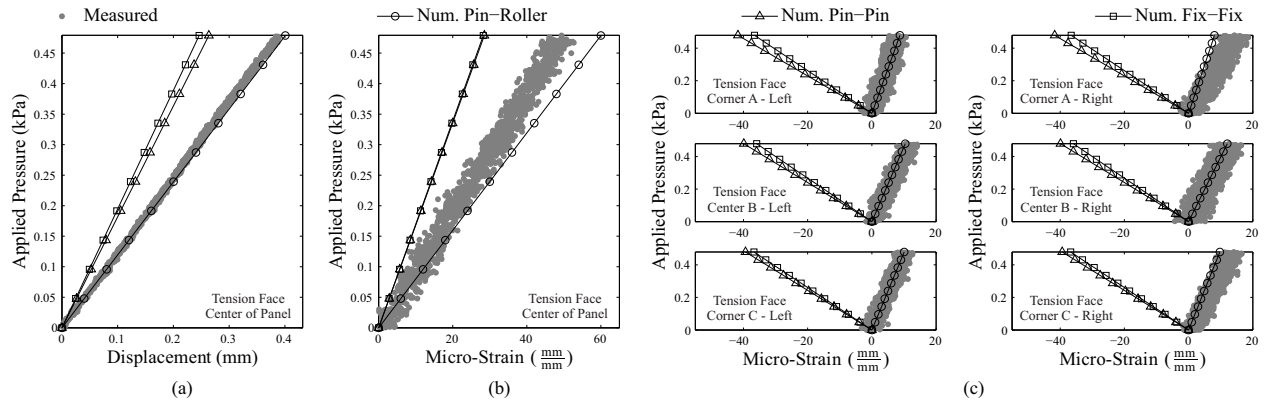


FIG. 3: Measured and numerical behavior (tension face) for hinges in compression test: (a) global displacements (center of panel), (b) global longitudinal strains (center of panel), and (c) localized longitudinal strains near hinges.

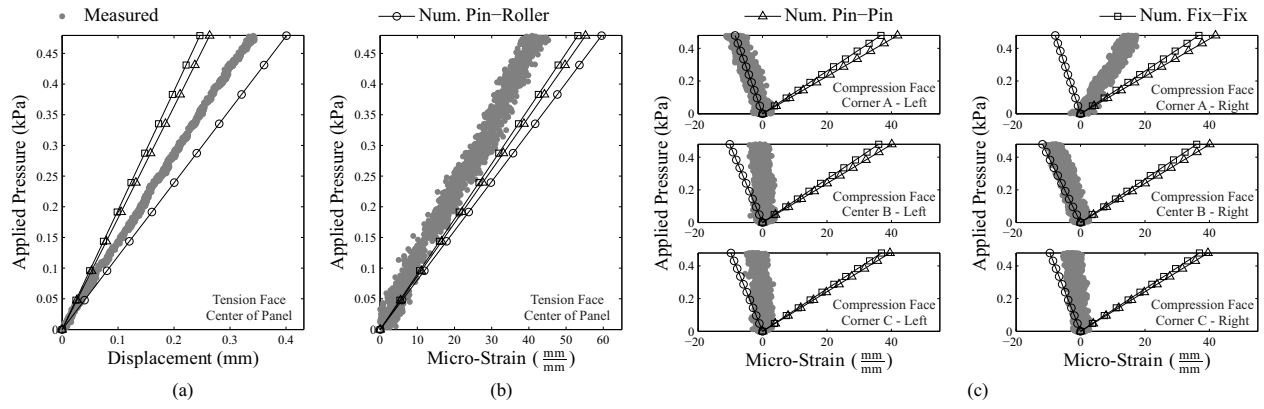


FIG. 4: Measured and numerical behavior for hinges in tension test: (a) global displacements (tension face, center of panel), (b) global longitudinal strains (tension face, center of panel), and (c) localized longitudinal strains near hinges (compression face).

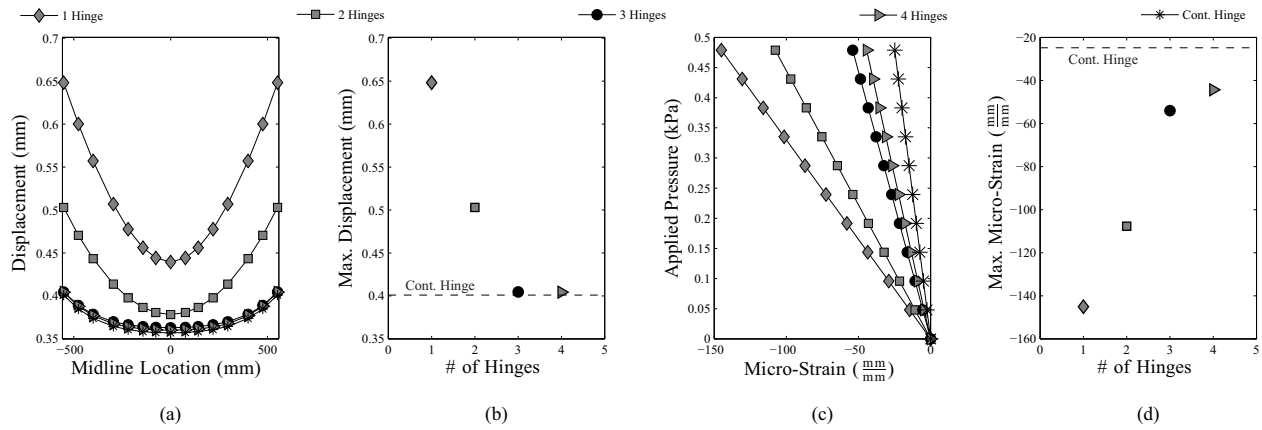


FIG. 5: Numerical behavior of panel (tension face) with varying number of hinged connectors: (a) global displacements along panel midline at an applied pressure of 0.479 kPa, (b) maximum displacement at an applied pressure of 0.479 kPa, (c) longitudinal strain near hinge, and (d) maximum longitudinal strain near hinge at an applied pressure of 0.479 kPa. Reprinted from Ballard et al. (2015), permission request pending.

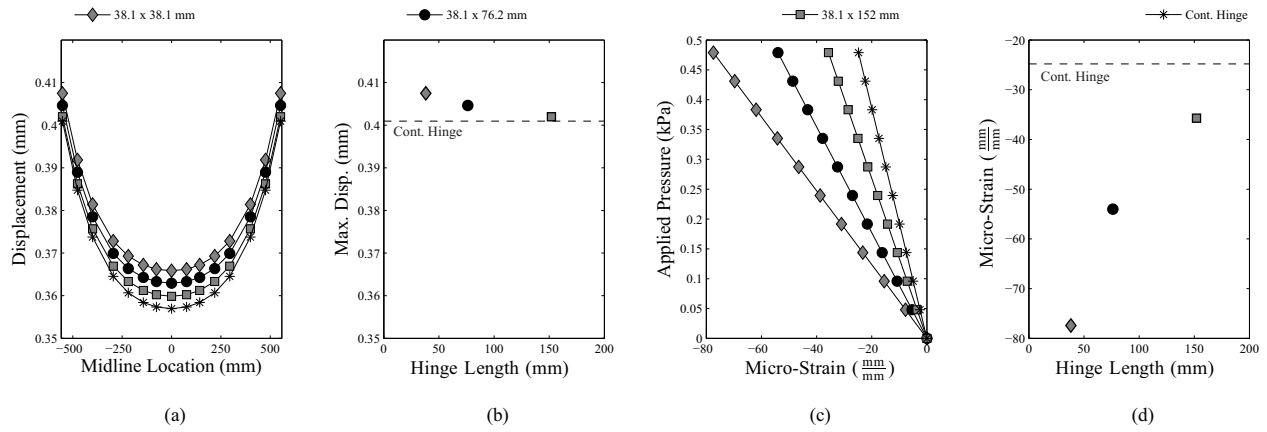


FIG. 6: Numerical behavior of panel (tension face) with varying length of hinged connectors: (a) global displacements along panel midline at an applied pressure of 0.479 kPa, (b) maximum displacement at an applied pressure of 0.479 kPa, (c) longitudinal strain near hinge, and (d) maximum longitudinal strain near hinge at an applied pressure of 0.479 kPa. Reprinted from Ballard et al. (2015), permission request pending.

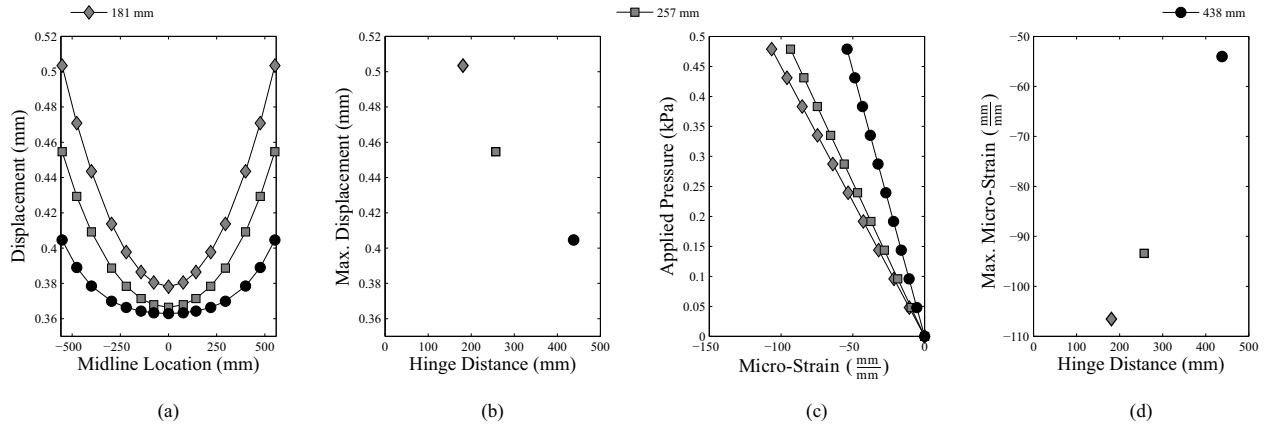


FIG. 7: Numerical behavior of panel (tension face) with varying hinge placement: (a) global displacements along panel midline at an applied pressure of 0.479 kPa, (b) maximum displacement at an applied pressure of 0.479 kPa, (c) longitudinal strain near hinge, and (d) maximum longitudinal strain near hinge at an applied pressure of 0.479 kPa. Reprinted from Ballard et al. (2015), permission request pending.